# DOCUMENT 00 90 00 ADDENDUM

# ADDENDUM NO. [2] Date: January 4, 2021

- RE: INDEPENDENCE SCHOOL DISTRICT EAST ELEMENTARY SCHOOL REMODEL 1103 1<sup>ST</sup> ST W WEST ELEMENTARY SCHOOL ADDITION 1301 1<sup>ST</sup> ST W INDEPENDENCE, IOWA 50644 HSR PROJECT NO. 19045
- FROM: HSR Associates, Inc 100 Milwaukee Street La Crosse, WI 54603 (608) 784-1830

# To: Prospective Bidders

This addendum forms a part of the Contract Documents and modifies the original Bidding Documents dated December 2020. Acknowledge receipt of this Addendum in the space provided on the bid form. Failure to do so may subject the Bidder to disqualification.

This Addendum consists of [3] pages and [9] 30 x 42 Drawings.

# CHANGES TO BIDDING REQUIREMENTS AND CONDITIONS OF THE CONTRACT:

1. Contractors desiring to be listed on our website as plan holders can contact Toni at HSR. tfurlano@hsrassociates.com.

# **CHANGES TO SPECIFICATIONS:**

- 2. Section 07 72 00 ROOF ACCESSORIES
  - a. Roof hatch size shall be 36 x 36 inches.
- 3. Section 08 43 13 ALUMINUM FRAMED STOREFRONT
  - a. 2.04, D: Operable sash shall be equal to or better than storefront framing performance.
- 4. Section 09 84 30 SOUND ABSORBING WALL AND CEILING UNITS
  - a. 2.01, A. Clarification: Quantities and layout shall be determined by supplier based on specified information in this section. 4 x 4 ceiling panel layout shall be coordinated with ceiling system supplier from Section 09 51 00. Walls in each room are clear with the exception of doors, windows, casework and marker boards as indicated on floor plan.

# 5. Section 09 91 00 INTERIOR PAINTING

a. Existing interior metal lockers to be painted: Lockers surfaces inside and out shall be washed/wiped to remove dirt and residue. Surfaces shall be sanded to roughen for paint adhesion. At chip areas taper edges to eliminate site lines. Mask numbers and other items that are currently not painted. Paint shall be applied with a fine tip airless sprayer. All surfaces inside and out shall be painted. Paint Type: Water based alkyd specifically recommended for this type of painted metal surface. Mask and protect surrounding floor, wall and ceiling surfaces from over spray.

# 6. Section 23 08 00 COMMISSIONING OF HVAC

- a. Commissioning is required at West Elementary only. Responsibility for commissioning is stated in 1.05, C.
- 7. <u>Section 23 09 93 Sequence of Operations</u>
  - a. 3.07 Exhaust Fan Control
    - i. Classroom exhaust (RX-2 thru RX-6) shall have BAS timer to automatically cycle off exhaust after 1-hour (adjustable) of operation.
  - b. 3.09 Pressure Independent VAV Control
  - c. Classroom VAV boxes shall be commanded to maximum airflow when associated classroom exhaust fan is energized.

# **CHANGES TO DRAWINGS:**

- 8. <u>West Elementary: Sheet A090 REMOVAL PLANS</u> 30 x 42 attached hereto
  - a. Revisions clouded on Drawing.
  - b. Stoop removal at south entry added.
- 9. West Elementary: Sheet A100 FIRST FLOOR REMODEL PLAN 30 x 42 attached hereto
  - a. Revisions clouded on Drawing.
  - b. Extents of existing locker painting identified.
- 10. West Elementary: Sheet A120 ROOF PLAN No Drawing Reissued
  - a. Roof hatch moved south 2 feet to align with bar joist spacing. Field verify final location. Provide cricket as required at north edge of hatch to divert water.
- 11. <u>West Elementary: Sheet S001 STRUCTURAL NOTES</u> 30 x 42 attached hereto a. Revisions clouded on Drawing.
- 12. <u>West Elementary: Sheet S002 STRUCTURAL SCHEDULES</u> 30 x 42 attached hereto a. Revisions clouded on Drawing.
- 13. West Elementary: Sheet S100 FOUNDATION PLAN 30 x 42 attached hereto
  - a. Revisions clouded on Drawing.'
- 14. West Elementary: Sheet S130 ROOF FRAMING PLAN 30 x 42 attached hereto
  - a. Revisions clouded on Drawing.
  - b. Control joint locations added to CMU bearing walls.

# 15. West Elementary: Sheet S500 FOUNDATION DETAILS 30 x 42 attached hereto

- a. Revisions clouded on Drawing.
- 16. West Elementary: Sheet S501 FRAMING DETAILS 30 x 42 attached hereto
  - a. Revisions clouded on Drawing.
- 17. <u>East Elementary: Sheet A600 DOOR SCHEDULE, FRAME ELEVATION AND WALL TYPES</u> No Drawing Reissued
  - a. Detail 2A600: Change glass type to GLT-4.
- 18. East Elementary: Sheet S100 PLANS AND DETAILS 30 x 42 attached hereto
  - a. Revisions clouded on Drawing.
  - b. Detail 7S100 revised.

# **PRIOR APPROVALS**

- 1. Section 08 43 13 ALUMINUM FRAMED STOREFRONT
  - a. EFCO 403X and 406X
- 2. Section 09 84 30 SOUND ABSORBING WALL AND CEILING UNITS
  - a. Kinetics Noise Control
- Section 10 51 13METAL LOCKERS

   Olympus; Hercules All-Welded Locker.
- 4. <u>Section 23 21 16 Hydronic Specialties 2.01 Balancing Stations</u> a. Griswold
- 5. Section 23 21 17 Air Control Devices 2.03 Coalescing Type Air Separator
  - a. Taco
  - b. B&G
- 6. Section 23 33 00 Air Duct Accessories
  - a. Tamco
  - b. United Enertech
- 7. Section 23 33 19 Sound Attenuators
  - a. Commercial Acoustics
  - b. Kinetics Noise Control
- 8. <u>Section 23 36 00 Air Terminal Units</u>
  - a. Krueger
- 9. Section 23 37 50 Metal Louvers-Stationary a. United Enertech
- 10. Section 23 73 13 Modular Air Handling Units
  - a. Aaon
  - b. Carrier
- 11. Section 23 74 13 Packaged Gas Electric Rooftop Units
  - a. LG
  - b. Carrier

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# FIRST FLOOR REMODEL PLAN 1/8" = 1'-0"





**GENERAL NOTES:** 

A SEE ID SHEETS FOR FLOOR AND WALL FINISH LAYOUTS.

# **KEY NOTES PLAN** CONCRETE STOOP- SEE STRUCTURAL. LADDER TO ROOF- SEE 2A311.





<ul> <li>BUILDING CODES</li> <li>DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE PROVIDE</li> </ul>	/ISIONS OF THE 2018 INTERNATIONAL BUILDING	<ul> <li>SYSTEM NOTES</li> <li>FOUNDATIONS AND EARTHWORK</li> <li>PEMOVE EXISTING SUBJICAL TOP SOIL AND VEGETATION FROM WITHIN THE</li> </ul>
RISK CATEGORY	III	FEET BEYOND. EXCAVATE MATERIAL TO PROPOSED SLAB-ON-GRADE SUBG TIRED VEHICLE. SOILS WHICH HEAVE, PUMP, OR DO NOT READILY COMPACT WITH ENGINEERED FILL.
<ul> <li>DESIGN LOADS AND DATA</li> <li>SUPERIMPOSED LOADS STAIR DEAD</li> </ul>	5 PSF	SUBGRADE PREPARATION FOR FOOTINGS SHALL CONSIST OF EXCAVATION CAPACITY SOILS AT OR NEAR DESIGN FOOTING ELEVATIONS. WHERE UNSU BEARINGDEPTH. SEE OVER EXCAVATION DETAIL.
LIVE MECHANICAL ROOMS DEAD	100 PSF	ALL COMPACTION REQUIREMENTS REFER TO % OF MAXIMUM DRY DENSITY GRANULAR STRUCTURAL FILL BENEATH FOOTINGS SHALL BE PLACED IN LA
MISCELLANEOUS (HVAC, PIPING, LIGHTS, CEILING ROOF LOADS DEAD	15 PSF 25 PSF	LAYER SHALL BE COMPACTED TO 95%. COHESIVE FILL APPROVED BY THE G PLACED IN LAYERS NO THICKER THAN 8", AND EACH LAYER SHALL BE COMP MATERIALS AS REQUIRED TO OBTAIN PROPER COMPACTION. COHESIVE SO
<ul> <li>ROOF LOADS GROUND SNOW (pg) ONOW DEMOITY</li> </ul>	20 PSF 30 PSF	SIGNIFICANT PERCENT OF COHESIVE FINES SHALL BE CONDITIONED TO WIT AT COMPACTION.
ROOF EXPOSURE SNOW IMPORTANCE FACTOR (I <sub>s</sub> )	PARTIALLY EXPOSED	FOR GENERAL INFORMATION AND SPECIFIC RECOMMENDATIONS AND REQU SITE, REFER TO THE PROJECT GEOTECHNICAL REPORT PREPARED BY CHO NUMBER 17488.20.IAW, DATED NOVEMBER 18, 2020.
THERMAL FACTOR - BUILDING (Ct) THERMAL FACTOR - CANOPY (Ct) ELAT ROOF SNOW LOAD (nt)	1.0 1.1 1.2 25.4 PSF	ALL ACTIVITIES CONCERNING PREPARATION AND VERIFICATION OF BEARING SHALL BE SUPERVISED AND APPROVED BY A QUALIFIED GEOTECHNICAL EN
DRIFT LOAD MECHANICAL EQUIPMENT, PIPING AND ROOF TOP AHU'S WIND DATA	AS NOTED ON DRAWINGS AS NOTED ON DRAWINGS	FOOTINGS ARE CENTERED ON WALLS ABOVE UNLESS NOTED OTHERWISE.
BASIC WIND SPEED (3 SECOND GUST) BUILDING ENCLOSURE EXPOSURE	116 MPH ENCLOSED C	BACKFILL AGAINST ANY STRUCTURAL ELEMENT UNTIL THAT ELEMENT HAS A DO NOT BACKFILL AGAINST BASEMENT WALLS UNTIL TOP AND BOTTOM OF A SI AB-ON-GRADE
WIND IMPORTANCE FACTOR (I w) WIND DIRECTIONALITY FACTOR (K d) TOPOGRAPHIC FACTOR (K zt)	1.0 0.85 1.0	TOP OF FOOTING ELEVATION NOTED ON DRAWINGS REPRESENT CONSIDER PROTECTION FROM FROST AND MINIMUM DEPTH TO SOILS CAPABLE OF PR
GUST FACTOR (BUILDING IS FLEXIBLE) (G f) INTERNAL PRESSURE COEFFICIENT (GC pi) ANALYSIS PROCEDURE	0.85 ± 0.18 CHAPTER 26 & CHAPTER 28	UNCERTAINTIES INHERENT IN DETERMINING THE ELEVATION OF SOILS ADE CAPACITY MAY REQUIRE FOUNDATIONS TO BE LOWERED – IN NO CASE SHA NOTED. A GEOTECHNICAL ENGINEER SHALL VERIFY THAT SOIL AT THE FOO
EDGE ZONE WIDTH (2a) MEAN ROOF HEIGHT (h) ROOF PLANE SLOPE (0)	12 FT 15 FT 1.2 DEGREES	<ul> <li>REQUIRED DESIGN SOIL BEARING CAPACITY.</li> <li>CAST-IN-PLACE CONCRETE</li> <li>DECION AND CONCEPTING SUM LEDE IN ACCORDANCE WITH THE PROVISION</li> </ul>
MINIMUM NET UPLIFT INTERIOR SPACES EXTERIOR CANOPIES/SOFFITS	15 PSF 30 PSF	WHERE MORE RESTRICTIVE REQUIREMENTS ARE NOTED.
<ul> <li>SEISMIC DATA SEISMIC IMPORTANCE FACTOR MAPPED SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIODS (S MAPPED SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIODS (S)</li> </ul>	1.25 s) .060	DRAWINGS. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH
MAPPED SPECTRAL RESPONSE ACCELERATION FOR T SECOND PERIOD ( SITE CLASS PER ASCE CHAPTER 20.1 DESIGN SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIODS (S DESIGN SPECTRAL RESPONSE ACCELERATION FOR 1 SECOND REPORT (	S <sub>1</sub> ) .049 D s) .065	#3 - #5 BARS #6 - #18 BARS CONCRETE NOT EXPOSED TO EARTH OR WEATHER
BASIC SEISMIC FORCE RESISTING SYSTEM AND PARAMETERS R = 20 $R = 20$ $C = 175$	B B	WALLS - #14 THRU #18 BARS
SEISMIC RESPONSE COEFFICIENT (C <sub>s</sub> ) ANALYSIS PROCEDURE	.04 MINIMUM EQUIVALENT LATERAL FORCE ANALYSIS	PROVIDE (2) #5 BARS AROUND ALL OPENINGS AND (2) #5 DIAGONAL BARS A CORNERS. BARS SHALL EXTEND A MINIMUM OF 24" PAST OPENING.
MATERIAL STRENGTHS AND STANDARDS		ALL BAR SPLICES SHALL BE CONTACT LAP SPLICED USING CLASS B TENSION STAGGERED A MINIMUM OF 3'-0" UNLESS DETAILED OTHERWISE.
THE MATERIAL STRENGTHS AND STANDARDS LISTED HERE REPRESENT A SE NOTED IN THE SPECIFICATIONS. SEE SPECIFICATIONS FOR ADDITIONAL INFO BETWEEN THESE NOTES AND THE SPECIFICATIONS, THESE NOTES SHALL GO	LECTED SUMMARY OF THE REQUIREMENTS RMATION. IN CASE OF DISCREPANCY VERN.	FIELD WELDING OF ASTM A615 REINFORCING STEEL IS NOT PERMITTED. FIE NOT PERMITTED EXCEPT WHERE SPECIFICALLY DETAILED ON STRUCTURAL
<ul> <li>SOILS DESIGN SOIL BEARING CAPACITY FOR SPREAD/STRIP FOOTINGS</li> <li>CONCRETE (28 DAY STRENGTH)</li> </ul>	3000 PSF	CORING OF COLUMNS, WALLS, BEAMS, JOISTS AND SLABS IS NOT PERMITTE PENETRATIONS AT ALL LOCATIONS APPROVED BY THE STRUCTURAL ENGIN
FOOTINGS, DRILLED PIERS, STEEL PILE FILL FOUNDATION WALLS INTERIOR SLAB-ON-GRADE	f`c = 3,000 PSI f`c = 4,000 PSI f`c = 4,000 PSI	<ul> <li>CONCRETE MASONRY DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE PROVISIONS EXCEPT WHERE MORE RESTRICTIVE REQUIREMENTS ARE NOTED.</li> </ul>
<ul> <li>EXTERIOR SLAB-ON-GRADE</li> <li>REINFORCING STEEL WELDED WIRE FABRIC, PROVIDED IN FLAT SHEETS ONLY (ASTM A185)</li> </ul>	$f_{c} = 4,500 \text{ PSI}$ $f_{y} = 65,000 \text{ PSI}$	ALL CMU SHALL BE PLACED IN RUNNING BOND. UNLESS NOTED OTHERWISE PRO REINFORCEMENT WITH 9 GAUGE SIDE AND CROSS RODS AT 16" OC VERTICALLY
<ul> <li>DEFORMED BARS (ASTM A615, GRADE 60)</li> <li>MASONRY SOLID CONCRETE BRICK (ASTM C55)</li> </ul>	f <sub>y</sub> = 60,000 PSI 3,500 PSI	CONTINUOUS UNOBSTRUCTED CELL FROM BOTTOM TO TOP OF BAR. CELL CON LESS THAN 3" X 4" IN PLAN AREA.
CONCRETE MASONRY UNIT ASSEMBLY CONCRETE MASONRY UNIT (ASTM C90) MORTAR (ASTM C270)	f m = 2,500 PSI 3250 PSI TYPE S	PORTIONS OF CMU CONSTRUCTION REQUIRING STRUCTURAL FILL SHALL USE G IN CMU CONSTRUCTION IS NOT PERMITTED. WHERE CLEARANCES AND CONGES WITH PEA GRAVEL AGGREGATE: OTHERWISE USE FINE GROUT
<ul> <li>ANCHOR RODS (ASTM F1554, GRADE 36)</li> <li>STRUCTURAL STEEL (SHAPES) WE WE SECTIONS (ASTM A992)</li> </ul>	$f_{y} = 36,000 \text{ PSI}$ $f_{y} = 36,000 \text{ PSI}$ $F_{y} = 50,000 \text{ PSI}$ $F_{y} = 65,000 \text{ PSI}$	REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION OF ALL VERTICAL CONTR PERIMETER WALLS AND FOR EXTERIOR WALLS.
M, S, HP SECTIONS (ASTM A332) M, S, HP SECTIONS, CHANNELS, ANGLES, PLATES (ASTM A36) HSS SHAPES – RECTANGULAR (ASTM A500, GRADE C) HSS SHAPES – ROUND (ASTM A500, GRADE C)	$F_y = 36,000 \text{ PSI}; F_u = 58,000 \text{ PSI}$ $F_y = 36,000 \text{ PSI}; F_u = 58,000 \text{ PSI}$ $F_y = 50,000 \text{ PSI}; F_u = 62,000 \text{ PSI}$ $F_u = 46,000 \text{ PSI}; F_u = 62,000 \text{ PSI}$	PROVIDE STEEL PIPE SLEEVES AT ALL LOCATIONS WHERE PIPING PASSES THRC
<ul> <li>STEEL PIPE (ASTM A53, GRADE B) PLATES (ASTM A36)</li> <li>STRUCTURAL STEEL (CONNECTIONS)</li> </ul>	$F_y = 35,000 \text{ PSI}; F_u = 60,000 \text{ PSI}$ $F_y = 36,000 \text{ PSI}; F_u = 58,000 \text{ PSI}$	WHERE BOND BEAMS INTERSECT AT WALL CORNERS AT DIFFERENT ELEVATION: CORNER FOR A MINIMUM OF TWO FULL BLOCK LENGTHS BEFORE TERMINATING. SAME WALL AT DIFFERENT ELEVATIONS, RUN BOND BEAMS PAST ONE ANOTHER
ANCHOR RODS (ÀSTM F1554, GRADE 36) HIGH STRENGTH BOLTS (1 1/2" MAXIMUM DIAMETER) WELDING ELECTRODES	F <sub>y</sub> = 36,000 PSI A325 AS NOTED E70XX	EENGTHS BEFORE TERMINATING     STRUCTURAL STEEL
SHEAR STUD CONNECTORS (ASTM A108, GRADE 1010 THROUGH 1020) DOWEL BAR ANCHORS (ASTM A496) THREADED RODS (ASTM A36)	F <sub>y</sub> = 50,000 PSI F <sub>y</sub> = 70,000 PSI F <sub>y</sub> = 36,000 PSI	DESIGN, DETAILING, AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE STEEL BUILDINGS AISC 360 -10, THE CODE OF STANDARD PRACTICE FOR STEEL AISC 303 -10,AND THE STEEL CONSTRUCTION MANUAL FOURTEENTH EDITION.
<ul> <li>GROUT (ASTM C1107)</li> <li>COLD-FORMED METAL FRAMING COLD-FORMED MATERIAL - 18 GAUGE AND THINNER (ASTM A653, GRADE COLD FORMED MATERIAL - 16 GAUGE AND THICKER (ASTM A653, GRADE</li> </ul>	$f_c = 5,000 \text{ PSI}$ 33) $f_y = 33,000 \text{ PSI}$	TYPICAL DETAILS INDICATE GENERAL CRITERIA FOR DESIGN AND DETAILING OF INTENDED TO CONVEY COMPLETE INFORMATION CONCERNING SIZE AND QUANT ANGLES, WELDS AND SIMILAR ITEMS THAT ARE DEVELOPED THROUGH THE DESI
ANCHOR RODS (ASTM F1554, GRADE 36) CONNECTOR PLATES (ASTM A36) CONNECTOR BOLTS (ASTM A36)	$f_y = 36,000 \text{ PSI}$ $f_y = 36,000 \text{ PSI}$ $f_y = 36,000 \text{ PSI}$ $F_y = 36,000 \text{ PSI}$	FOR A SPECIFIC SET OF LOADS AND COMBINATIONS. DETAILS THAT CONVEY SP ESTABLISH MINIMUM REQUIREMENTS AND ARE NOT INTENDED TO CONVEY A CO
WELDING ELECTRODES GALVANIZING THICKNESS	E60XX G60	UNLESS OTHERWISE NOTED, ALL STEEL TO STEEL FRAMING HAS BEEN SELECTE SHEAR ONLY, USING DOUBLE ANGLE OR DOUBLE BENT PLATE CONNECTIONS SH AND FIELD BOLTED TO SUPPORTING MEMBER WITH HIGH STRENGTH BOLTS IN B
<ul> <li>GENERAL NOTES</li> <li>EXISTING CONDITIONS</li> <li>INFORMATION PERTAINING TO EXISTING CONDITIONS GIVEN ON THE STERNART</li> </ul>	RUCTURAL DRAWINGS REPRESENTS THE	SYMMETRICAL ABOUT THE BEAM WEB. FABRICATORS PROPOSING TO USE ALTE NOT SPECIFICALLY DETAILED ON STRUCTURAL DRAWINGS SHALL SUBMIT ALTEF BIDDING, AND SHALL BEAR ALL COSTS ASSOCIATED WITH REVIEW, ENGINEERING
ACTUAL EXISTING FIELD CONDITION TO THE BEST OF OUR KNOWLEDGE. AS TO THEIR ACCURACY. CONTRACTOR SHALL FIELD VERIFY EXISTING E CONDITIONS AFFECTING THE WORK BY DIRECT SURVEY AND MEASUREM	R.A. SMITH, INC. MAKES NO WARRANTY ELEVATIONS, DIMENSIONS AND BUILDING IENT PRIOR TO THE FABRICATION, ERECTION	ALTERNATIVE CONNECTIONS SINGLE PLATE SHEAR TAB CONNECTIONS MAY BE USED IN LIEU OF DOUBLE AND
OR CONSTRUCTION OF ANY ITEM IMPACTED BY EXISTING CONDITIONS. CONTRACT DOCUMENTS AND FIELD CONDITIONS FOR REVIEW. ANY WOF OF THE DISCREPANCIES IS SUBJECT TO REMOVAL AND REPLACEMENT A	REPORT DISCREPANCIES BETWEEN THE RK PERFORMED PRIOR TO THE RESOLUTION T THE CONTRACTORS EXPENSE.	SIDE OF A SUPPORT MEMBER IS MATCHED BY A SIMILAR CONNECTION ON THE C MEMBER, AND WHERE BEAM SPANS DO NOT DIFFER BY MORE THAN 50% OF THE TABS MAY NOT BE USED FOR CONNECTION OF FRAMING MEMBERS TO COLUMN.
EXISTING STRUCTURE TO REMAIN IS SHOWN WITH LIGHT GRAY LINES. E NOT GENERALLY SHOWN ON STRUCTURAL DRAWINGS - SEE ARCHITECTI	XISTING STRUCTURE TO BE REMOVED IS JRAL DRAWINGS FOR DEMOLITION DRAWINGS.	MEMBERS UNLESS SPECIFICALLY DETAILED ON DRAWINGS.
ALL EXISTING STRUCTURE TO REMAIN TO BE SUPPORTED BY NEW CONS CONSTRUCTION IS IN PLACE, COMPLETED, AND CAPABLE OF SUPPORTIN	TRUCTION SHALL BE SHORED UNTIL NEW G THE EXISTING STRUCTURE. EXISTING	STRUCTURAL DRAWINGS SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN AND RETAINED BY THE FABRICATOR, USING THE REACTIONS SHOWN. IF NO REA CONNECTIONS SHALL BE DESIGNED FOR 50 % OF THE TOTAL UNIFORM LOAD CA
<ul> <li>IT IS NO LONGER AFFECTED BY CONSTRUCTION ACTIVITIES.</li> <li>CONSTRUCTION</li> </ul>		SIZE, SPAN AND GRADE OF STEEL. ALL MOMENT CONNECTIONS SHALL BE DESIGNED AND DETAILED BY AN ENGINE
UNLESS SPECIFICALLY NOTED OTHERWISE, BUILDING STRUCTURE HAS E CONDITION ONLY, AND HAS NOT BEEN ANALYZED, INVESTIGATED OR DES INDIVIDUAL MEMBER, STABILITY DURING CONSTRUCTION. CONTRACTOR	SEEN DESIGNED FOR THE FINAL COMPLETED SIGNED FOR OVERALL STRUCTURE, OR SHALL PROVIDE AND MAINTAIN TEMPORARY	WISCONSIN AND RETAINED BY THE FABRICATOR, USING THE REACTIONS AND MO REACTIONS AND MOMENTS ARE NOT SHOWN, CONNECTION SHALL BE DESIGNED THE BEAM IN MOMENT AND SHEAR.
EVERY STAGE OF CONSTRUCTION UNTIL THE FINAL COMPLETION OF THE STRUCTURE, WHILE UNDER CONSTRUCTION IS INTENDED TO BE STABLE TEMPORARY BRACES AND SUPPORTS, WHICH SHALL ADDITIONALLY PRO	E STRUCTURE. NO PORTION OF THE BUILDING IN THE ABSENCE OF THE CONTRACTORS	DESIGN OF STAIRS, HANDRAILS AND GUARDRAILS SHALL BE BY THE STEEL SUPP
LOADING. MATERIALS AND EQUIPMENT SHALL BE STORED, TRANSPORTE NOT EXCEED THE DESIGN FLOOR LOADING.	ED AND INSTALLED IN A MANNER THAT WILL	DRAWINGS. PROVIDE HOLES IN BEAMS TO ACCOMMODATE WOOD CONNECTIONS TO STEEL.
CONTRACTOR IS SOLELY RESPONSIBLE FOR THE MEANS, METHODS, TEC CONSTRUCTION INCLUDING, BUT NOT LIMITED TO, TEMPORARY BRACING SUPPORT IMPOSED CONSTRUCTION LOADS, AND OTHER SIMILAR ITEMS.	CHNIQUES, SEQUENCES AND PROCEDURES OF 6, SUPPORTS, SHORING, FORMING TO	BAR JOISTS     ALL STANDARD K. LH AND DLH SERIES JOISTS SHALL BE DESIGNED FOR A SHEAF
STRUCTURAL DOCUMENTS MAY REFER TO OSHA REQUIREMENTS. SUCH INTENDED TO IDENTIFY ALL APPLICABLE OSHA REQUIREMENTS.	REFERENCES ARE INCIDENTAL, AND ARE NOT	AND VARYING LINEARLY TO 25% OF THE REACTION AT THE MIDSPAN OF THE JOI POTENTIAL STRESS REVERSALS THE SHEAR CAPACITY OF THE JOIST SHALL BE DISTANCE BEYOND THE MIDSPAN EQUAL TO MINIMUM OF ONE PANEL WIDTH, RC
<ul> <li>COMPLETENESS INFORMATION CONTAINED IN THE GENERAL NOTES IS ONLY A PARTIAL S SEE SPECIFICATIONS. PLANS AND DETAILS FOR ADDITIONAL REQUIREMENT</li> </ul>	UMMARY OF PROJECT REQUIREMENTS.	WHERE JOISTS ARE DESIGNATED BY DEPTH, SERIES AND TOTAL LOAD / LIVE LO/ LOAD PLUS SELF WEIGHT OF JOIST AND IS THE RESPONSIBILITY OF THE JOIST S
USE ONLY DIMENSIONS INDICATED ON THE DRAWINGS. DO NOT MANUAL DIMENSIONS MEASURED FROM ELECTRONIC DRAWING FILES.	LY SCALE THE DRAWINGS OR USE ANY	WHERE JOIST DESIGNATION INCLUDES "SP", FINAL DESIGN SHALL BE PER LOADI WEIGHT OF JOIST AND IS THE RESPONSIBILITY OF THE JOIST SUPPLIER.
UNLESS NOTED OTHERWISE, CENTERLINE OF FLOOR FRAMING ELEMENT CENTERLINES, AND FRAMING ELEMENTS ARE EQUALLY SPACED BETWEE	S COINCIDES WITH COLUMN N ADJACENT COLUMN CENTERLINES.	WHERE STANDARD JOIST DESIGNATION FOR DEPTH, SERIES AND SIZE OCCURS DESIGN SHALL BE PER LOADING DIAGRAM PROVIDED PLUS SELF WEIGHT OF JOI
MAJOR OPENING LOCATIONS AND SIZES ARE INDICATED ON THE STRUCT AND SLEEVES REQUIRED TO ACCOMMODATE VARIOUS BUILDING SERVIC	URAL DRAWINGS - SMALLER OPENINGS ES MAY NOT BE NOTED. CONTRACTOR TO	JOIST SUPPLIER.
VERIFY THE SIZE AND LOCATION OF ALL ARCHITECTURAL, MECHANICAL, INCLUDING CLEARANCE REQUIREMENTS CONTAINED IN THE RESPECTIVE AND IN-PLACE OPERATION OF THE RESPECTIVE EQUIPMENT OR ITEMS.	ELECTRICAL AND PLUMBING OPENINGS, E DISCIPLINE DOCUMENTS FOR INSTALLATION JNDER NO CIRCUMSTANCES MAY	WHERE BRIDGING INTERFERES WITH MECHANICAL OR OTHER TRADE INSTALLAT BRIDGING AFTER METAL DECK IS COMPLETE IN PLACE, UPON RECIEPT OF WRIT
WITHOUT WRITTEN APPROVAL OF THE STRUCTURAL ELEMENT AFTER FINAL PL	ACEMENT IN THE BUILDING STRUCTURE,	BRIDGING REMOVED SHALL BE REPLACED AS DIRECTED BY THE ENGINEER, INCL BRACING AS MAY BE NECESSARY IN THE SOLE JUDGEMENT OF THE ENGINEER.
SHEETS FOR LOCATIONS AND DIMENSIONS OF PADS, CURBS, EQUIPMEN REGLETS, REVEALS, FINISHES AND OTHER MISCELLANEOUS PROJECT RE INCIDENTAL ACCOMMODATION BY THE RUIL DING STRUCTURE BUT APE N	T SUPPORTS, DEPRESSIONS, INSERTS, DRIPS, EQUIREMENTS THAT NECESSITATE OT SHOWN ON THE STRUCTURAL DRAWINGS	NO FIELD DRILLED HOLES OR CUTS ARE PERMITTED IN ANY JOIST CHORD OR WI MAXIMUM HANGER LOAD TO BE LOCATED ALONG BAR JOIST TOP CHORD BETWE
GENERAL     THE STRUCTURE HAS BEEN DESIGNED AS UNRESTRAINED FOR THE PUR     ASSEMBLY EVALUATIONS	POSE OF FIRE RATING AND FIREPROOFING	ALL CONCENTRATED LOADS EXCEEDING 100 POUNDS SHALL BE APPLIED AT A JO INDICATED ON LOAD DIAGRAMS AND CHORDS HAVE BEEN SPECIFICALLY DESIGN
STRUCTURAL COMPONENTS HAVE NOT BEEN DESIGNED FOR VIBRATORY	( EQUIPMENT UNLESS NOTED OTHERWISE.	UNLESS SUPPLEMENTAL CHURD BRACING IS PROVIDED. SUPPLEMENTAL CHORE DETAILED ON THE DRAWINGS BY THE CONTRACTOR RESPONSIBLE FOR THE CO PANEL POINTS.
LATERAL BRACING FOR NON-STRUCTURAL FLEMENTS DESIGNED AND DE	TAILED BY COMPONENT SUPPLIERS SHALL BE	JOISTS AND SEAT CONNECTIONS SHALL BE DESIGNED TO RESIST AXIAL LOADS I FORCE ACTING PARALLEL TO THE JOIST NOT LESS THAN 5% OF THE (DEAD + LIV GREATER
DESIGNED TO APPLY LOADS DIRECTLY TO FLOOR OR ROOF DIAPHRAGM BOTTOM FLANGES OF BEAMS OR BOTTOM CHORDS OF JOISTS UNLESS 1 ADDITIONAL BRACING FROM THOSE ELEMENTS TO THE FLOOR OR ROOF	S. BRACES SHALL NOT ATTACH DIRECTLY TO THE COMPONENT SUPPLIER PROVIDES DIAPHRAGM AT EACH ATTACHMENT POINT.	WHERE FIRE PROTECTION LINE RUNS PARALLEL TO A BAR JOIST, LINES UP TO A A SINGLE JOIST. LINES LARGER THAN 4" SHALL BE HUNG BETWEEN BAR JOISTS I
HOLES, NOTCHES, BLOCK-OUTS AND OTHER SIMILAR FIELD MODIFICATION SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS OR APPROVED	INS TO STRUCTURAL MEMBERS NOT D SHOP DRAWINGS ARE NOT PERMITTED.	UNLESS SPECIFICALLY NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS, MA SIZE FIRE PROTECTION LINE MAY NOT BE MORE THAN 15'.
EXCEPT AS NOTED BELOW, ALL FUTURE EXPANSION IS ASSUMED TO BE GRAVITY AND LATERAL LOADS.	COMPLETELY SELF SUPPORTING FOR BOTH	

EGETATION FROM WITHIN THE BUILDING AREA AND A MINIMUM OF TEN DSED SLAB-ON-GRADE SUBGRADE. PROOFROLL WITH A HEAVY RUBBER R DO NOT READILY COMPACT SHALL BE EXCAVATED AND REPLACED

LL CONSIST OF EXCAVATION TO REQUIRED ALLOWABLE BEARING SELEVATIONS. WHERE UNSUITABLE SOIL IS ENCOUNTERED AT NOMINAL

6 OF MAXIMUM DRY DENSITY PER ASTM D-1557 MODIFIED PROCTOR. NGS SHALL BE PLACED IN LAYERS NO MORE THAN 8" THICK, AND EACH VE FILL APPROVED BY THE GEOTECHNICAL CONSULTANT SHALL BE EACH LAYER SHALL BE COMPACTED TO 95%. MOISTURE CONDITION FILL COMPACTION. COHESIVE SOILS OR GRANULAR SOILS WITH A ALL BE CONDITIONED TO WITHIN 3% OF OPTIMUM MOISTURE CONTENT

COMMENDATIONS AND REQUIREMENTS PERTAINING TO THE PROJECT REPORT PREPARED BY CHOSEN VALLEY TESTING, INC, JOB ND VERIFICATION OF BEARING SOILS FOR SLAB-ON-GRADE AND FOOTINGS

UALIFIED GEOTECHNICAL ENGINEER. CENTERED ON GRIDLINES UNLESS NOTED OTHERWISE. CONTINUOUS INLESS NOTED OTHERWISE.

DATION WALLS, GRADE BEAMS AND OTHER SIMILAR ELEMENTS. DO NOT I UNTIL THAT ELEMENT HAS ATTAINED FULL DESIGN STRENGTH. UNTIL TOP AND BOTTOM OF WALL IS BRACED BY FLOOR FRAMING AND

INGS REPRESENT CONSIDERED ENGINEERING JUDGMENTS ABOUT TH TO SOILS CAPABLE OF PROVIDING DESIGN SOIL BEARING CAPACITY. E ELEVATION OF SOILS ADEQUATE TO PROVIDE DESIGN BEARING LOWERED – IN NO CASE SHALL TOP OF FOOTING BE HIGHER THAN ERIFY THAT SOIL AT THE FOOTING BASE IS ADEQUATE TO PROVIDE THE

ORDANCE WITH THE PROVISIONS OF ACI 318 –11 EXCEPT RE NOTED.

ED BELOW UNLESS SPECIFICALLY NOTED OTHERWISE ON STRUCTURAL NTLY EXPOSED TO EARTH 3"

THER		
	1 1/2"	
	2"	
WEATHER		
	3/4"	
	1 1/2"	

ND (2) #5 DIAGONAL BARS AT ALL OPENING AND RE-ENTRANT 24" PAST OPENING.

CED USING CLASS B TENSION LAP LENGTHS, WITH ADJACENT LAPS LED OTHERWISE.

TEEL IS NOT PERMITTED. FIELD BENDING OF REINFORCING STEEL IS Y DETAILED ON STRUCTURAL DRAWINGS. AND SLABS IS NOT PERMITTED. PROVIDE STEEL SLEEVES FOR ALL

BY THE STRUCTURAL ENGINEER PRIOR TO PLACING CONCRETE.

ANCE WITH THE PROVISIONS OF ACI 530 -11 AND ACI 530.1 - 11 'S ARE NOTED. ESS NOTED OTHERWISE PROVIDE CONTINUOUS LADDER TYPE RODS AT 16" OC VERTICALLY IN ALL WALLS AND PIERS, AND AT 8"

ARS ARE REQUIRED, CONSTRUCT CMU WALL TO PROVIDE A 1 TO TOP OF BAR. CELL CONTAINING A SINGLE BAR SHALL NOT BE

RUCTURAL FILL SHALL USE GROUT ONLY. USE OF CONCRETE FILL E CLEARANCES AND CONGESTION PERMIT, USE COARSE GROUT INE GROUT. ION OF ALL VERTICAL CONTROL JOINTS IN EXTERIOR WYTHES OF

WHERE PIPING PASSES THROUGH CMU WALL

S AT DIFFERENT ELEVATIONS, RUN EACH BOND BEAM AROUND THE GTHS BEFORE TERMINATING. WHERE BOND BEAMS ADJOIN ON THE BEAMS PAST ONE ANOTHER A MINIMUM OF FOUR FULL BLOCK

IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL DARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES AL FOURTEENTH EDITION.

R DESIGN AND DETAILING OF CONNECTIONS. THEY ARE NOT ONCERNING SIZE AND QUANTITY OF CONNECTORS, PLATES, ELOPED THROUGH THE DESIGN OF AN INDIVIDUAL CONNECTION S. DETAILS THAT CONVEY SPECIFIC COMPONENT INFORMATION INTENDED TO CONVEY A COMPLETE DESIGN UNLESS NOTED.

RAMING HAS BEEN SELECTED ASSUMING ATTACHMENTS FOR ENT PLATE CONNECTIONS SHOP WELDED TO FRAMING MEMBER HIGH STRENGTH BOLTS IN BEARING. CONNECTIONS SHALL BE RS PROPOSING TO USE ALTERNATIVE METHODS OF ATTACHMENT WINGS SHALL SUBMIT ALTERNATIVE FOR CONSIDERATION DURING WITH REVIEW, ENGINEERING REDESIGN, AND APPROVAL OF

ISED IN LIEU OF DOUBLE ANGLE OR DOUBLE BENT PLATE RAWINGS OR WHERE CONNECTION OF FRAMING MEMBER TO ONE IILAR CONNECTION ON THE OPPOSITE SIDE OF THE SAME SUPPORT R BY MORE THAN 50% OF THE LARGER SPAN. SINGLE PLATE SHEAR MING MEMBERS TO COLUMNS OR TO SPANDREL (EDGE) SUPPORT AWINGS

S AND GIRDERS NOT SHOWN OR COMPLETELY DETAILED ON THE N ENGINEER REGISTERED IN THE STATE OF WISCONSIN ACTIONS SHOWN. IF NO REACTION IS SHOWN, BEAM HE TOTAL UNIFORM LOAD CAPACITY FOR THE GIVEN MEMBER

ND DETAILED BY AN ENGINEER REGISTERED IN THE STATE OF SING THE REACTIONS AND MOMENTS SHOWN. WHERE NECTION SHALL BE DESIGNED TO DEVELOP THE FULL CAPACITY OF

HALL BE BY THE STEEL SUPPLIER.

LANEOUS STRUCTURAL STEEL NOT NOTED ON STRUCTURAL

L BE DESIGNED FOR A SHEAR CAPACITY EQUAL TO THE REACTION, AT THE MIDSPAN OF THE JOIST. IN ORDER TO ACCOUNT FOR TTY OF THE JOIST SHALL BE MAINTAINED AT THE 25% VALUE FOR A JM OF ONE PANEL WIDTH, ROUNDED UP TO THE NEXT PANEL POINT.

S AND TOTAL LOAD / LIVE LOAD, FINAL DESIGN SHALL BE PER NOTED PONSIBILITY OF THE JOIST SUPPLIER. DESIGN SHALL BE PER LOADING DIAGRAM PROVIDED PLUS SELF THE JOIST SUPPLIER.

I, SERIES AND SIZE OCCURS PRIOR TO THE DESIGNATION "SP", FINAL D PLUS SELF WEIGHT OF JOIST, SHALL AT A MINIMUM USE THE DEPTH AND SERIES NOTED, AND IS THE RESPONSIBILITY OF THE

T UTILIZE A 1/3 STRESS INCREASE.

OR OTHER TRADE INSTALLATION. CONTRACTOR MAY REMOVE CE, UPON RECIEPT OF WRITTEN APPROVAL FROM THE ENGINEER. CTED BY THE ENGINEER, INCLUDING ADDITIONAL SUPPLEMENTAL GEMENT OF THE ENGINEER.

) IN ANY JOIST CHORD OR WEB MEMBER. AR JOIST TOP CHORD BETWEEN PANEL POINTS IS 100 POUNDS.

DS SHALL BE APPLIED AT A JOIST PANEL POINT UNLESS LOADS ARE BEEN SPECIFICALLY DESIGNED FOR CONCENTRATED LOADS, OR DED. SUPPLEMENTAL CHORD BRACING SHALL BE PROVIDED AS R RESPONSIBLE FOR THE CONCENTRATED LOADS NOT APPLIED AT

ED TO RESIST AXIAL LOADS INDICATED, OR RESIST A HORIZONTAL THAN 5% OF THE (DEAD + LIVE) LOAD REACTION, WHICHEVER IS

A BAR JOIST, LINES UP TO AND INCLUDING 4" MAY BE SUPPORTED BY IUNG BETWEEN BAR JOISTS USING TRAPEZE HANGER. STRUCTURAL DRAWINGS, MAXIMUM SPACING OF HANGERS ON ANY AN 15'.

SYSTEM NOTES (CONTINUED)

 METAL DECKING PROVIDE ANGLE SUPPORTS FOR METAL DECK AT ALL COLUMN FACES WHERE SUPPORT IS REQUIRED, AND IS NOT PROVIDED BY MEMBERS FRAMING TO COLUMN. ANGLE FRAMING SHALL BE A MINIMUM OF L2x2x3/16. NO LOADS FROM ARCHITECTURAL, MECHANICAL, ELECTRICAL OR PLUMBING ITEMS, SINGLY OR IN AGGREGATE, IN EXCESS OF 25 POUNDS SHALL BE HUNG FROM METAL ROOF DECK IN ANY 4 SQUARE FOOT AREA. LOADS EXCEEDING THIS LIMIT REQUIRE SUPPLEMENTAL FRAMING ATTACHED DIRECTLY TO STRUCTURAL FRAMING.

SPLICES AT CONTINUOUS DIAPHRAGM CHORD ANGLES SHALL BE FULL PENETRATION WELDS UNLESS NOTED.

CONDUIT AND SLEEVES IN CONCRETE

THE USE OF ALUMINUM CONDUITS EMBEDDED IN STRUCTURAL CONCRETE ELEMENTS (WALLS) IS PROHIBITED. WHERE SPECIFICALLY APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO THE PLACEMENT OF SLEEVES, CONDUIT OF ANY TYPE MAY PASS PERPENDICULARLY THROUGH A STRUCTURAL CONCRETE FLEMENT PROVIDED THAT A SCHEDULE 40 STEEL SLEEVE IS PROVIDED WITH AN INSIDE DIAMETER NO LESS THAN 1" LARGER THAN THE CONDUIT OUTSIDE DIAMETER. APPROVAL WILL GENERALLY NOT BE GIVEN FOR CONDUIT GROUPS WITH A COMBINED DIAMETER GREATER THAN 12" AT ONE LOCATION, UNLESS SPECIFICALLY INCORPORATED BY REFERENCE IN THE DRAWINGS.

CONDUITS EMBEDDED IN STRUCTURAL CONCRETE ELEMENTS, SHALL SATISFY THE FOLLOWING CRITERIA: - THEY ARE UNCOATED OR GALVANIZED IRON OR STEEL NOT THINNER THAN STANDARD SCHEDULE 40 STEEL PIPE. - THEY SHALL NOT BE LARGER IN OUTSIDE DIAMETER THAN 1/3 THE OVERALL THICKNESS OF THE SLAB, WALL OR BEAM IN WHICH THEY ARE EMBEDDED, OR 4" OUTSIDE DIAMETER, WHICHEVER IS SMALLER. FOR SLABS-ON-METAL DECK, THICKNESS SHALL BE THE CONCRETE DEPTH ABOVE FLUTES. - SPECIFIED CONCRETE COVER FOR PIPES, CONDUITS AND FITTINGS SHALL NOT BE LESS THAN 2" FOR CONCRETE EXPOSED TO EARTH OR WEATHER, NOR LESS THAN 1" FOR CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND. - MULTIPLE CONDUITS SHALL NOT BE CLOSELY GROUPED. WHERE IT IS DESIRED TO PLACE MULTIPLE CONDUITS CLOSELY TOGETHER, INDIVIDUAL CONDUITS SHALL NOT BE SPACED CLOSER THAN FOUR OUTSIDE DIAMETERS OF THE LARGEST CONDUIT IN THE GROUP ON CENTER. NO MORE THAN FOUR (4) CONDUITS MAY BE PLACED IN A GROUP. CONDUIT GROUPS SHALL BE SEPARATED BY A MINIMUM CLEAR DISTANCE OF 30 INCHES. - CONDUITS MAY NOT BE STACKED VERTICALLY. - PIPING AND CONDUIT SHALL BE FABRICATED AND INSTALLED SO THAT CUTTING, BENDING OR DISPLACEMENT OF REINFORCEMENT OR OTHER EMBEDMENTS FROM THEIR PROPER LOCATION WILL NOT BE REQUIRED. - DO NOT TIE CONDUIT TO REINFORCEMENT STEEL. PROVIDE A MINIMUM OF 2" CLEARANCE FOR CONCRETE FLOW BETWEEN CONDUIT AND REINFORCEMENT STEEL.

 POST-INSTALLED ANCHORAGE ALL POST-INSTALLED ANCHORS MUST BE INSTALLED IN STRICT CONFORMANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS INCLUDING, BUT NOT LIMITED TO, DRILL TYPE, HOLE CLEANING, INSTALLATION TORQUE, AND TEMPERATURE CONSTRAINTS.

ALL PERSONNEL INSTALLING POST-INSTALLED ANCHORS SHALL BE TRAINED BY THE MANUFACTURER ON PROPER INSTALLATION TECHNIQUE. CONTRACTOR SHALL COORDINATE ANY ON-SITE TRAINING WITH THE ANCHOR MANUFACTURER. TRAINING DOCUMENTATION SHALL BE AVAILABLE UPON REQUEST.

WHEN A SPECIFIC PRODUCT AND MANUFACTURER IS REFERENCED IN THE CONTRACT DOCUMENTS, THAT SPECIFIC PRODUCT SHALL BE USED UNLESS NOTED OTHERWISE. BELOW CONTAINS A LIST OF PRE-APPROVED ANCHORS FOR USE AS AN EQUAL (WHERE "OR EQUAL" IS INDICATED) OR WHERE POST-INSTALLED ANCHORAGE IS REFERRED TO IN THE DOCUMENTS GENERICALLY (E.G. "ADHESIVE ANCHOR").

PROVIDE SPECIAL INSPECTION FOR ALL POST-INSTALLED ANCHORS PER THE EVALUATION REPORT OR AS INDICATED OTHERWISE. THE ANCHOR MANUFACTURER'S REPRESENTATIVE SHALL BE PRESENT DURING THE INITIAL INSTALLATION OF EACH TYPE OF ANCHOR TO REVIEW AND APPROVE THE CONTRACTOR'S INSTALLATION PROCEDURES. CONCRETE ANCHORS

PRIOR TO INSTALLING POST-INSTALLED ANCHORS, CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF OF 2,500 PSI AND BE 21 DAYS OLD. ADHERE TO MANUFACTURER'S REQUIREMENTS FOR REQUIRED INSTALLATION TEMPERATURES AND HOLE CONDITION (WET, DRY, SATURATED).

EXPANSION ANCHORS FOR USE IN CONCRETE INCLUDE: HILTI: KWIK-BOLT TZ SIMPSON STRONG-TIE: STRONG-BOLT 2

DEWALT/POWERS: POWER-STUD+SD2 SCREW ANCHORS FOR USE IN CONCRETE INCLUDE:

HILTI: HUS-EZ SIMPSON STRONG-TIE: TITEN HD DEWALT/POWERS: SCREW-BOLT+

ADHESIVE ANCHORS FOR USE IN CONCRETE INCLUDE: HILTI: HIT-RF 500 V3 OR HIT-HY 200 SIMPSON STRONG-TIE: SET-XP OR AT-XP DEWALT/POWERS: PURE110+ OR AC200+ GOLD

DO NOT USE ADHESIVE ANCHORS IN OVERHEAD APPLICATIONS UNLESS SPECIFICALLY INDICATED ON THE CONTRACT DOCUMENTS. FOR ADHESIVE ANCHORS INSTALLED HORIZONTALLY OR UPWARDLY INCLINED, INSTALLER SHALL HOLD AN ACTIVE ACI/CRSI ISSUED ADHESIVE ANCHOR INSTALLER CERTIFICATION IN ADDITION TO TRAINING BY THE ANCHOR MANUFACTURER. CONTINUOUS SPECIAL INSPECTION FOR ADHESIVE ANCHORS INSTALLED AT THESE ANGLES IS REQUIRED. THE SPECIAL INSPECTOR SHALL PROVIDE A REPORT TO THE STRUCTURAL ENGINEER OF RECORD INDICATING THAT THE MATERIALS USED AND INSTALLATION PROCEDURES CONFORM WITH THE CONSTRUCTION DOCUMENTS AND MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ALL OTHER POST-INSTALLED ANCHORS SHALL HAVE PERIODIC SPECIAL INSPECTION AT A MINIMUM UNLESS MORE STRINGENT REQUIREMENTS ARE INDICATED IN THE SPECIFIC ANCHOR'S EVALUATION REPORT.

• MASONRY ANCHORS INSTALLATION OF POST-INSTALLED ANCHORAGE INTO GROUTED CELLS SHALL BE MADE ONCE GROUT HAS REACHED A COMPRESSIVE STRENGTH OF 2,000 PSI. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR ALL POST-INSTALLED ANCHORAGE INTO MASONRY. IF MORE

STRINGENT REQUIREMENTS ARE INDICATED IN THE SPECIFIC ANCHOR'S EVALUATION REPORT, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN.

EXPANSION ANCHORS TO SOLID OR GROUTED CMU INCLUDE: HILTI: KWIK-BOLT 3 SIMPSON STRONG-TIE: STRONG-BOLT 2

DEWALT/POWERS: POWER-STUD+SD1 SCREW ANCHORS TO SOLID OR GROUTED CMU INCLUDE:

HILTI: KWIK-HUS-EZ SIMPSON STRONG-TIE: TITEN HD DEWALT/POWERS: SCREW-BOLT+

DEWALT/POWERS: AC100+ GOLD

ADHESIVE ANCHORS TO SOLID, GROUTED, OR HOLLOW CMU AND UNREINFORCED BRICK INCLUDE: HILTI: HIT-HY 70 SIMPSON STRONG-TIE: SET-XP (CMU ONLY) SIMPSON STRONG-TIE: AT (BRICK ONLY)



SNOW DRIFT PLAN





ISOMETRIC



STRUCTURAL SHEET INDEX

S001 STRUCTURAL NOTES S002 STRUCTURAL SCHEDULES S100 FOUNDATION PLAN S130 ROOF FRAMING PLAN S500 FOUNDATION DETAILS S501 FRAMING DETAILS



MISCELLANEOUS LINTEL SCHEDULE (SEE NOTE 1)							
WALL THICKNESS	CLEAR MASONRY OPENING WIDTH	SECTIO	DN				
ALL	AT FIRE EXTINGUISHER CABINETS AND DRINKING FOUNDATIONS	1/4" PL	_				
4"	UP TO 4'-0"	L3 1/2x3 1/2x3/8	L				
4"	UP TO 8'-0"	L5x3 1/2x3/8	L				
6"	UP TO 5'-0"	(2) L3 1/2x2 1/2x1/4					
6"	UP TO 7'-0"	WT 4 x 10.5	$\perp$				
6"	UP TO 9'-0"	WT 7 x 11	$\perp$				
8"	UP TO 5'-0"	(2) L3 1/2x3 1/2x1/4	JL				
8"	UP TO 7'-0"	(2) L4x3 1/2x5/16 LLV	JL				
8"	UP TO 9'-0"	WT 7 x 15	$\perp$				
8"	UP TO 4'-0"	8" HIGH x 8" WIDE BOND BEAM w/ (2) #5 x CONT	i. J				
8"	UP TO 8'-0"	16" HIGH x 8" WIDE BOND BEAM w/ (2) #5 x CONT					
12"	UP TO 4'-0"	8" HIGH x 12" WIDE BOND BEAM w/ (2) #5 x CONT	[]				
12"	UP TO 8'-0"	16" HIGH x 12" WIDE BOND BEAM w/ (2) #5 x CONT					

LINTEL NOTES:

1. LINTELS CALLED OUT IN THIS SCHEDULE ARE FOR NON-LOAD BEARING MASONRY WALLS AND FOR LOAD BEARING WALLS WHERE LOAD IS INTRODUCED ABOVE THE LINTEL AT A DISTANCE GREATER THAN THE LINTEL SPAN.

2. PROVIDE MINIMUM 8" BEARING AT EACH END OF LINTEL. 3. CENTER LINTELS IN WALL UNLESS NOTED OTHERWISE.

4. BOTTOM PLATES UNDER WIDE FLANGE SHAPES SHALL BE EXTENDED TO THE FULL LENGTH OF LINTEL.

5. WELD LINTEL COMPONENTS INTO SINGLE UNIT.

FRAMES HAVE A HEADSPAN OF 4'-0" OR LESS.

6. NO LINTELS REQUIRED FOR 4" AND 6" NON-LOAD BEARING MASONRY WALLS WHERE GROUTED HOLLOW METAL

7. PROVIDE THESE LINTELS WHERE OTHER LINTELS ARE NOT SPECIFICALLY DETAILED.

8. GROUT BLOCK CORES SOLID MINIMUM (3) COURSES BELOW LINTEL BEARING.





CONNECTIONS SHOWN ARE MINIMUM CONNECTIONS UNLESS NOTED OTHERWISE.

5. CONNECTION ANGLES SHALL BE 36 ksi MINIMUM.

6. ALL STEEL EXPOSED TO EXTERIOR CONDITIONS SHALL BE GALVANIZED. 7. ALL STANDARD DOUBLE ANGLE CONNECTION SHALL BE IN ACCORDANCE WITH AISC STEEL CONSTRUCTION MANUAL, 13th EDITION & SHALL BE TYPE 2 FRAMING, UNO.

<b></b>												
	UNCOATED TENSION DEVELOPMENT & CLASS "B" LAP SPLICE SCHEDULE (f'c = 3,000 psi)											
	TENSION DEVELOPMENT LENGTH CLASS "B" TENSION LAP LENGTH											
BAR	CLR CC	)V = .75"	CLR C	CLR COV = 1" CLR		)V = 1.5"	" CLR COV = .75"		CLR C	OV = 1"	CLR COV = 1.5"	
SIZE	BOT BARS	TOP BARS	BOT BARS	TOP BARS	BOT BARS	TOP BARS	BOT BARS	TOP BARS	BOT BARS	TOP BARS	BOT BARS	TOP BARS
#3	12	13	12	13	12	13	13	17	13	17	13	17
#4	17	22	13	17	13	17	22	28	17	23	17	23
#5	24	32	20	26	17	22	32	41	26	33	22	28
#6	33	43	27	35	20	26	43	56	35	46	26	34
#7	53	69	44	57	33	43	69	90	57	74	43	55
#8	66	86	55	72	41	54	86	111	72	93	54	70
#9	80	104	67	87	51	66	104	135	87	113	66	86
#10	96	125	81	106	62	81	125	162	106	137	81	105
#11	113	146	96	125	74	97	146	190	125	162	97	125
<u>SC</u> 1) E	HEDULE BASED C	NOTES: N:										
	1a	1. GRADE	E 60 REIN		MENT B	ARS.						
	10	). NORW/	ARS IN V	VALLS A	ND SLAF	BS.						
2) T	OP BAR	S ARE H	IORIZON	ITAL BAF	RS WITH	MORE T	HAN 12"	OF				
3)[	CONCRETE BELOW THE BARS.											
5,1	3) FUR LIGHT WEIGHT CUNCRETE, MULTIPLY TABLED VALUES BY 1.33.											
THI	IS SCHE	DULE IS	PROVID	ED FOR	THE CO			THE				
	AWINGS	SHALL (	CLEARL				ED LAP L	LENGTH	SHUP S.			

	UNCOATED TENSION DEVELOPMENT &									
	CLASS "B" LAP SPLICE SCHEDULE (fc = 4,000 psi									
	TENSION DEVELOPMENT LENGTH							ASS "B" <sup>-</sup>	TENSION	N LA
BAR	CLR COV = .75"		CLR COV = 1"		CLR COV = 1.5"		CLR COV = .75"		CLR COV	
SIZE	BOT BARS	TOP BARS	BOT BARS	TOP BARS	BOT BARS	TOP BARS	BOT BARS	TOP BARS	BOT BARS	T B/
#3	12	12	12	12	12	12	12	15	12	1
#4	15	19	12	15	12	15	19	24	15	2
#5	21	28	17	22	15	19	28	36	22	2
#6	29	37	24	31	17	22	37	48	31	4
#7	46	60	38	50	28	37	60	78	50	6
#8	57	74	48	62	36	47	74	96	62	8
#9	69	90	58	76	44	57	90	117	76	9
#10	83	108	70	92	54	70	108	140	92	1
#11	98	127	83	108	64	84	127	165	108	1
<u>SCHE</u> 1) BA	SCHEDULE NOTES: 1) BASED ON: 1a. GRADE 60 REINFORCEMENT BARS. 1b. NORMAL WEIGHT CONCRETE.									

1c. FOR BARS IN WALLS AND SLABS.

2) TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF

CONCRETE BELOW THE BARS. 3) FOR LIGHTWEIGHT CONCRETE, MULTIPLY TABLED VALUES BY 1.33.

THIS SCHEDULE IS PROVIDED FOR THE CONVENIENCE OF THE CONTRACTOR AND IS NOT INTENDED TO COVER ALL SITUATIONS. SHOP DRAWINGS SHALL CLEARLY INDICATE ALL REQUIRED LAP LENGTHS.



L	INTEL SCHE	DULE						
	SECTION	END BEARING PLATES	REMARKS					
		N/A	1,3,4,5					
		N/A	2,7,8					
		N/A						
		PL3/8"x5"x0'-8" W/(2) 1/2" DIA x 6" LONG HWS	12					
		PL3/8"x7"x0'-8" W/(2) 1/2" DIA x 6" LONG HWS						
$\checkmark$		PL3/8"x7"x0'-8" W/(2) 1/2" DIA x 6" LONG HWS	$\frown \frown$					
UTE	D CELL, EACH END OF L	INTEL, TYP.						
		IREMENTS.	-	<				
RING	ED OTHERWISE; AT EACH END OF LINTI	EL.		Z				
UNLE	ESS NOTED OTHERWIS							
JALLI ETAI	ED FOR SHALL EXTEND	VALL OPENING REINFORCEN		<				
ETAI	LS FOR TYPICAL CMU (	CONTROL JOINT REQUIREME	NTS	2				
) PLA	CE CMU.							
NELD INTF	DED STUDS (HWS) AT 24	4" OC ON TOP OF LINTEL. GF	ROUT CMU	Ϊ				
CHOF	RS AT 16" OC FACH SIDI	F OF WFB.		)				
OTTO	OM PLATES) TO BE HOT	- OF THESE GAI VANIZED						
אדר מ	ΩF WΔ11							
			_					
JLE F	OR BRICK SUPPORT IN	I FRONT OF CMU LINTELS.						
RED	TO MATCH LINTEL BEAK	RING ELEVATION						
SONRY WALL								
	DEFINES OVE OF WALL REIN TO OCCUR	R WHAT LENGTH IFORCEMENT IS						
	CMU SIZE							
		R OF BARS						
	BAR SIZE (#4	ETC)						
	LOCATION OF	BAR(S)						
	WITHIN BLOCK	CORE:						
	EF - 1/2 TOT BARS F	TAL NUMBER OF						
	C - CENTE	R IN WALL						
	IF - INSIDE	FACE (NOTE 1)						
-		(INCHES)						
INDICATES WALL TO BE GROUTED SOLID. IF "G" IS OMITTED, GROUT BLOCK CORES AT BAR LOCATIONS ONLY								
INDICATES GROUT SOLID BELOW GRADE								
2	4G GB≻—	——————————————————————————————————————						
-								
/								
	NOTES:							
	1. MAINTAIN 1 INSIDE SUR	FACE OF BLOCK						





CONTINUOUS FOOTING SCHEDULE								
	CONTINUOUS FOOT	ING DIMENSIONS						
MARK	WIDTH	THICKNESS	FOOTING REINFORCEMENT	REMARKS				
W20	2'-0"	12"	(2) #5; B, CONT					

1. B = BOTTOM, T = TOP, LW = LONG WAY, SW = SHORT WAY, EW = EACH WAY.

2. ALL REINFORCEMENT BARS TO BE BOTTOM BARS UNLESS NOTED OTHERWISE.



12" CMU WALL



<u>8" CMU WALL</u>



TYPICAL CMU REINFORCEMENT AT OPENINGS

DOOR

1'-4"

GREATER

\_ \_ \_ \_

OPENINGS GREATER THAN 16" WIDE

MAX

<u>openings</u> <u>To 16"</u> <u>Wide</u>

 $\sim$   $\sim$   $\sim$ 

\_ \_ \_



<u>12" CMU WALL</u>



CLOSED TIE SETS AT 8" OC

(6) REINFORCE VERTICAL CELLS

AT EDGE OF WALL AND ADJACENT TO CONTROL

JOINTS

REINFORCEMENT

AT 16" OC TYPICAL

CORNER/END OF

- STEM WALL TOP

REINFORCEMENT

- FLOOR LINE

WALL

FOUNDATION LEGEND
CONCRETE PAD FOOTING
COLUMN FOOTING MARK
TOP OF WALL FOOTING ELEVATION
TOP OF LEDGE ELEVATION
TOP OF WALL ELEVATION
F40 99'-0"   P1   100'-0"
WALL FOOTING STEP MARKER
SLAB-ON-GRADE JOINT
TOP OF EXISTING WALL FOOTING ELEVATION - 96'-0"
MASONRY WALL AND CONCRETE FOOTING
MEMBER SIZES OR MARKS WITH A

FRAMING KEY NOTES

(1) FIELD CUT EXISTING SLAB FOR NEW LOAD BEARING CMU. FIELD VERIFY SLAB REMOVAL WIDTH, 2'-0" EAGH SIDE OF NEW WALL, MIN. ₩ V ) INFILL EXISTING WALL OPENING WITH AN 8" CONCRETE WALL #4 BARS AT 18" OC BW. DOWEL BARS INTO EXISTING CONCRETE FOUNDATION WALL 6" MIN. SEE ARCH FOR WATER PROOFING.  $\sim$ REMOVE EXISTING WALLS AND SLABS

OTHERWISE.

- NOTED OTHERWISE.
- SLAB ELEVATION.

















![](_page_12_Figure_0.jpeg)

![](_page_12_Figure_1.jpeg)

![](_page_12_Figure_2.jpeg)

	LINTEL SCHEDULE								
LINTEL MARK	DESCRIPTION	SECTION	END BEARING PLATES	REMARKS					
ML	REFER TO MISCELLANEOUS LINTEL SCHEDULE								
L1	W10x26 W/ BOTTOM PL 3/8"x7"		N/A	NORTH END - L4x4x3/8x0'-6" WELD TO COLUMN SOUTH END - DETAIL 1/S100 WITH TEFLON BEARING 1,2,3,10					
L2	W8x18 W/ BOTTOM PL 3/8"x7"		PL3/8"x7"x0'-7" W/(2) 1/2" DIA x 4" LONG HWS	1,2,3,10					
L3	W8x18 W/ BOTTOM PL 3/8"x11"	1/4 3-9	PL3/8"x7"x0'-8" W/(2) 1/2" DIA x 6" LONG HWS	1,2,3,10					
NOTES									

![](_page_12_Picture_16.jpeg)